FATIGUE AND STRENGTH TESTS
OF HEAT-STRAIGHTENED FERRY LOADING
BRIDGE HANGER BARS

by

Jeffrey W. Berman
Associate Professor

Vince Chaijaroen
Structures Lab Manager

Department of Civil and Environmental Engineering
University of Washington, Box 352700
Seattle, Washington 98195

Washington State Transportation Center (TRAC)
University of Washington, Box 354802
University District Building
1107 NE 45th Street, Suite 535
Seattle, Washington 98105-4631

Washington State Department of Transportation Technical Monitor
Jeri Bernstein
Bridge Engineer, Washington State Ferries

Prepared for
The State of Washington
Department of Transportation
Lynn A. Peterson, Secretary

June 2013
FATIGUE AND STRENGTH TESTS OF HEAT-StraIGHTENED FERRY LOADING BRIDGE HANGER BARS

Jeffrey W. Berman, Vince Chaizaroen

Washington State Transportation Center (TRAC)
University of Washington, Box 354802
University District Building; 1107 NE 45th Street, Suite 535
Seattle, Washington 98105-4631

Research Office
Washington State Department of Transportation
Transportation Building, MS 47372
Olympia, Washington 98504-7372
Project Manager: Rhonda Brooks, 360-705-7945

This study was conducted in cooperation with the U.S. Department of Transportation, Federal Highway Administration.

Tests were conducted on heat-straightened and/or bent live load hanger bars used in loading bridges in the Washington State Ferry (WSF) system. Both fatigue and ultimate strength tests were conducted. The study found that when heat-straightened three times, the hanger bars have a fatigue life that exceeds their design life. The data indicated that additional heat-straightening may be possible without concern for reducing the fatigue life. The yield strength of the hanger bars was found to be unaffected by either heat-straightening or by initial bending deformations. In both cases the hanger bar yield strength exceeded nominal values. The ultimate strength was somewhat reduced by the presence of initial bending deformation.
Disclaimer

The contents of this report reflect the views of the authors, who are responsible for the facts and the accuracy of the data presented herein. The contents do not necessarily reflect the official views or policies of the Washington State Department of Transportation or Federal Highway Administration. This report does not constitute a standard, specification, or regulation.
CONTENTS

Executive Summary ................................................................. vii
Introduction, Motivation and Objectives ........................................ 1
Fatigue Testing Set-Up and Results ............................................. 4
Ultimate Strength Testing Set-Up and Results .............................. 6
Conclusions .................................................................................. 11
Laboratory Disclaimer Statement ................................................ 12
Appendix ....................................................................................... 13
FIGURES AND TABLES

Figure 1. Loading Bridge Cross-Section with Hanger Bars Labeled .................................. 1
Figure 2. Loading Bridge Elevation with Hangar Bar Labeled ........................................... 2
Figure 3. Hanger Bar Detail .............................................................................................. 3
Figure 4. (a) Fatigue Test Set-Up and (b) Fatigue Specimen ........................................... 4
Figure 5. Ultimate Strength Test Set-Up (a) Nominal 10-ft Specimen and (b) 40-in. Specimen .............................................................................................................. 6
Figure 6. (a) Example of Typical Slotted Hole Yielding and Deformation and (b) Example of Typical net Section Fracture ............................................................................ 7
Figure 7. Load vs. Time Curves for Ultimate Strength Tests ............................................ 9
Table 1. Details and Yield and Ultimate Strengths of Tested Hanger Bars ....................... 10
EXECUTIVE SUMMARY

Objectives
The objective of this study was to determine whether heat-straightened and/or bent ferry loading bridge hanger bars have adequate fatigue life and ultimate strength.

Background
The bridges used to load vehicles onto Washington State ferries are supported on one end by hanger bars. These bars carry bridge loads in tension but can buckle in compression as the ferry rises with rising tides while at the dock. Washington State Ferries (WSF) engineers heat-straighten the buckled bars and return them to service. However, it is unclear whether the bars can be heat-straightened three times and safely reused. It is also unclear to WSF engineers what the ultimate tensile capacity of the plastically buckled bars is.

Research Activities
Two sets of tests were conducted on heat-straightened hanger bars. First, bars that had been heat-straightened three times were tested under fatigue loading with the amplitude of the varying loading near the design load for the bars, determined by the live truck loads on the bridge. Second, several hanger bars, heat-straightened two or three times or cold bent to 5 degrees, were tested in tension to failure to determine their ultimate strength.

Conclusions
Fatigue tests demonstrated that hanger bars heat-straightened three times have a fatigue life of at least 3 million cycles at a load range of 50 kips (10 kips tension to 60 kips tension). The ultimate strength tests demonstrated that the bars were able to reach the yield capacity of the net section regardless of the heat straightening or initial out-of-straightness. Ultimate hanger bar strength was not affected by heat-straightening, but initial out-straightness did reduce the ultimate capacity slightly. Results from all tests indicated that, for loads within the range used for testing, bars may be safely heat-straightened at least three times—and likely more—and returned to service.
The bridges used to load vehicles onto Washington State ferries are supported on one end by hanger bars. Figures 1, 2, and 3 show a typical loading bridge and hanger bar. These bars carry bridge loads in tension, but they can buckle in compression as the ferry rises with rising tides while at the dock or when workers adjust the bridge without removing the pins. Washington State Ferries (WSF) engineers then heat-straighten the bars and return them to service. However, it is unclear whether the bars can be heat-straightened three times and safely reused.

The objective of this research was to determine whether heat-straightened ferry loading bridge hanger bars have adequate fatigue life and ultimate strength. To achieve this objective, the
researchers carried out fatigue and ultimate strength tests in the University of Washington (UW) Structural Research Laboratory (SRL).

Figure 2. Loading Bridge Elevation with Hanger Bar Labeled.
Figure 3. Hanger Bar Detail
FATIGUE TESTING SETUP AND RESULTS

Test Set-Up

Two hanger bar specimens were tested under fatigue loading in a 110-kip fatigue test frame in the SRL. The test set-up is shown in Figure 4a. The specimens were 40 in. long and had three of the oval shaped holes shown in Figure 3, with 4 in. of overhang on each end. A typical specimen is shown in Figure 4b. Each specimen had been heat-straightened three times, and they were named 3A and 3B. The specimens were connected to the test frames by using a series of plates and pins. The pins that were used in bearing against the hanger bars were identical to those used in the ferry loading bridges to ensure that the stress distribution in the tests closely matched that expected in the field.

Figure 4. (a) Fatigue Test Setup (b) Fatigue Specimen

Mayes Testing Inc. conducted magnetic particle testing on both the fatigue specimens before and after testing to look for cracks. Some surface cracks were noted before testing but were likely the result of corrosion and did not grow during the tests. The inspection reports from Mayes Testing Inc. are included in the Appendix.
Both specimens were subjected to sinusoidal cyclic fatigue loading with peaks at 60 kips and 10 kips of tension at a rate of 3 Hz. The loading was conducted around the clock, and emergency switches were utilized to sense a failure and stop the hydraulic system. This loading protocol was agreed to by the SRL staff and the WSF engineers. They determined that the specimens should be subjected to 3 million cycles of loading, at which point the tests would be stopped if no failure occurred.

**Experimental Results**

Both specimens were loaded to 3 million cycles without failure. Post-test magnetic particle inspections conducted by Mayes Testing Inc. showed no signs of cracking at the hanger bar net section or in the regions of heat-straightening. Both specimens were then reused in the ultimate strength tests described below. Although twice heat-straightened specimens were also prepared, they were not tested in fatigue because the specimens that had been straightened three times performed well. Instead, they were tested for ultimate strength as described below. The design life for these bars is 10 years, and according to WSF engineers, the design life results in a loading of 1.3 million cycles at the tested stress range. Therefore, the experimental results indicated that the bars heat-straightened three times have ample fatigue life for their intended design life.
ULTIMATE STRENGTH TESTING SET-UP AND RESULTS

Test Set-up

Ultimate strength tests were conducted at the top of the SRL’s 2.4-million-pound capacity Universal Testing Machine (UTM). Two lengths of specimens were tested: (1) nominally 10-ft-long bars, one that had been heat-straightened multiple times and two that were installed bent to simulate their condition after buckling, and (2) 40-in.-long bars that were used (or designed to be used) in the fatigue test set-up. Figure 5 shows the two types of specimens installed in the UTM. The material for all bars was either A36 or an unknown older steel.

![Figure 5. Ultimate Strength Test Setup (a) Nominal 10 ft Specimen (b) 40 in. Specimen](image)

The set-up utilized pin connections at each end of the hanger bars, and the pins used were identical to those used in the field. Loading was applied slowly to each specimen and continued
until failure. Only the load was recorded during the tests. However, the tests were conducted under displacement control, i.e., the displacement of the UTM crosshead was used to control loading during the test. This crosshead displacement was applied at a uniform, slow rate. Thus, plotting the force applied versus time would indicate when the specimens began to yield, since there is a linear relationship between time and displacement.

**Experimental Results**

Each specimen exhibited reasonable ductility before fracturing either at a net section area adjacent to a slotted hole within the length of the specimen or at the net section where the pins connected to the specimens. After the tests, signs of yielding at all net section areas adjacent to the slotted holes were visible. An example is shown in Figure 6a. An example of the typical net section fracture that occurred after significant inelastic deformation is shown in Figure 6b.

![Figure 6. (a) Example of typical slotted hole yielding and deformation. (b) Example of typical net section fracture.](image-url)
Table 6 lists the details of each specimen. Specimens Ult1 and Ult2 were 10 ft long and were tested after being bent (Figure 5a). Specimen Ult3 had been heat-straightened three times. Specimens F3A and F3B were 40 in. long and had been subjected to 3 million cycles of fatigue loading as described above. Specimens F2A and F2B were 40 in. long and had been heat-straightened twice. As noted above, all specimens exhibited ductile behavior, and the two bent specimens did not fracture at the bend but rather at a different net section (at the pin connection or one slotted hole away from the pin connection).

Figure 7 shows the force versus time curves for each tested specimen, where the names correspond to the information in Table 1. As noted above, a constant rate of crosshead displacement was used to control the test so that the load versus time curves would be similar to load deformation curves. Figure 7 shows that each specimen had a clear yield point at which the slope of the curve changed. This was followed by ductile inelastic deformation and eventual fracture at a net section adjacent to a slotted hole. Note that the two long bent specimens showed a smaller initial stiffness as the bend in the bar was straightened. Also note that those two specimens had slightly lower ultimate strengths than the other specimens, although the yield strength was similar.

By using the curves of Figure 7, the yield and ultimate strengths of each specimen were determined, and the results are given in Table 1. The table also shows the nominal yield and ultimate strengths, assuming a yield stress of 36 ksi and an ultimate stress of 58 ksi each, times the net area at a long slotted hole of 9.5 in$^2$. A comparison of the nominal and experimentally obtained values indicated that the inelastic buckling deformation, heat-straightening (up to three times), and fatigue loading generally did not affect the yield strengths of the bars. The data in Table 1 do indicate that the bars tested in the bent configuration had somewhat lower ultimate strengths, producing effective ultimate stresses of 56.7 for Specimen Ult1 and 60 ksi for Specimen Ult2. The former was slightly lower than the minimum ultimate stress for A36 of 58 ksi. It is unclear why these specimens exhibited a lower ultimate strength, especially given that the fractures occurred far from the bends. Heat-straightening and fatigue loading were not found to affect the ultimate strength.
Figure 7. Load vs. Time Curves for Ultimate Strength Tests
<table>
<thead>
<tr>
<th>Specimen ID</th>
<th>Specimen Length</th>
<th>Specimen Details</th>
<th>Yield Strength¹ (kips)</th>
<th>Ultimate Strength² (kips)</th>
</tr>
</thead>
<tbody>
<tr>
<td>F2A</td>
<td>40 in.</td>
<td>Heat-straightened two times, loaded only for ultimate strength, A36 steel</td>
<td>408</td>
<td>651</td>
</tr>
<tr>
<td>F2B</td>
<td>40 in.</td>
<td>Heat-straightened two times, loaded only for ultimate strength, A36 steel</td>
<td>405</td>
<td>638</td>
</tr>
<tr>
<td>F3A</td>
<td>40 in.</td>
<td>Heat-straightened three times, loaded in fatigue before ultimate strength, A36 steel</td>
<td>402</td>
<td>661</td>
</tr>
<tr>
<td>F3A</td>
<td>40 in.</td>
<td>Heat-straightened three times, loaded in fatigue before ultimate strength, A36 steel</td>
<td>404</td>
<td>668</td>
</tr>
<tr>
<td>Ult1</td>
<td>10 ft.</td>
<td>Bent bar at approximately 10°, loaded only for ultimate strength, A36 steel</td>
<td>407</td>
<td>539</td>
</tr>
<tr>
<td>Ult2</td>
<td>10 ft.</td>
<td>Bent bar at approximately 10°, loaded only for ultimate strength, A36 steel</td>
<td>402</td>
<td>570</td>
</tr>
<tr>
<td>Ult3</td>
<td>10 ft.</td>
<td>Heat-straightened, loaded only for ultimate strength, unknown steel</td>
<td>430</td>
<td>698</td>
</tr>
<tr>
<td>Nominal</td>
<td>-</td>
<td>36 ksi yield stress, 58 ksi ultimate stress</td>
<td>342</td>
<td>551</td>
</tr>
</tbody>
</table>

¹ Yield strength estimated as point of significant change in slope in Figure 7 plots.
² Maximum force obtained during test.
CONCLUSIONS

The testing program demonstrated that hanger bars heat-straightened up to three times are able to

- resist 3 million cycles of fatigue loading with a range of 50 kips
- develop good ductility
- achieve yield strengths consistent with the yield stress of the original material, and
- achieve ultimate strengths consistent with the tensile stress of the original material.

Hanger bars with large bends from inelastic buckling are also able to achieve yield strengths consistent with the yield stress of the material, but they were observed in one case to have an ultimate strength slightly lower than what would be expected by developing the tensile stress of the material over the net section area.
LABORATORY DISCLAIMER STATEMENT:

The Structural Research Laboratory provides commercial testing services. These services are limited to testing and data collection. The results are valid at the time the test occurs on the specific specimens tested. The engineering response of similar items is not within the scope of the testing agreement. The SRL staff, the Department of Civil and Environmental Engineering, the College of Engineering, and the University of Washington disclaim any and all liability for any personal or property damage or loss as a result of use of the test results.
APPENDIX

Below are inspection reports from Mayes Testing Engineers, Inc. They performed magnetic particle testing on the hanger bars before fatigue loading (inspection on April 2, 2013) to check for initial cracks and following fatigue loading (inspection on May 17, 2013) for Specimen F3A and F3B (denoted 3A and 3B in the inspections reports). As shown, no indications of significance were found. Small indications parallel to the direction of the applied load were found, but they were not due to the fatigue loading applied but instead may have been related to corrosion or material imperfections.
Performed magnetic particle examination of four load bar plates, 2A, 2B, 3A, and 3B. This inspection was performed to determine if there were any flaws prior to fatigue testing. Noted flaw in bottom of plate 3A at X end. Notified Jeff Berman and Vince with UW.

![Image of a crack](image-url)

Fig. 1. Crack noted in bottom of 3A plate at X end

To the best of our knowledge, all items inspected today are in conformance with approved plans and specifications.

Inspector: Skip Szurek

Reviewed By:

Michael S. Dolder, P.E.
Vice President
### Nondestructive Examination Report

**Project No.:** L13023  
**Date:** 4/9/13  
**Project:** WSDOT Live Load Hangar Bar  
**Type of Inspection:** ☒ MT  ☐ PT

- **NDE Procedure:** MTE-AWS  
- **Revision No.:** 0  
- **Acceptance Standard:** AWS D1.1

- **Material Type:** Carbon Steel  
- **Surface Condition:** ☒ As Welded  ☐ Machined  ☐ Ground

- **Material Temp.:** 50's  
- **Heat Treatment:** ☐ Before  ☐ After

- **Magnetic Particle**  
  - **Equip. Manufacturer:** Parker  
  - **Model:** B300  
  - **Serial No.:** 16766  
  - **Cal Date:** 1/13  
  - **Current:** ☒ AC  ☐ DC  
  - **Amperage:** 6A  
  - **Inspection Method:** ☒ Dry  ☐ Wet  ☒ Visible  ☐ Fluorescent  
  - **Prod Spacing:** 4"  
  - **Test Method:** ☐ Solvent Removable  ☐ Visible  
  - **Dowel Time:**  
  - **Development Time:**  
  - **Water Washable:** ☐  
  - **Fluorescent:** ☐

### Weld or Part No.

<table>
<thead>
<tr>
<th>Part No.</th>
<th>Location</th>
<th>Accept</th>
<th>Reject</th>
<th>Remarks</th>
</tr>
</thead>
<tbody>
<tr>
<td>2A</td>
<td>four sides of plate</td>
<td>☒</td>
<td>☒</td>
<td>informational inspection</td>
</tr>
<tr>
<td>2B</td>
<td>four sides of plate</td>
<td>☒</td>
<td>☒</td>
<td>informational inspection</td>
</tr>
<tr>
<td>3A</td>
<td>top and both sides</td>
<td>☒</td>
<td>☒</td>
<td>informational inspection</td>
</tr>
<tr>
<td>3B</td>
<td>bottom</td>
<td>☒</td>
<td>☒</td>
<td>5&quot; longitudinal crack at X end</td>
</tr>
<tr>
<td></td>
<td>four sides of plate</td>
<td>☒</td>
<td>☐</td>
<td>informational inspection</td>
</tr>
</tbody>
</table>

- **No. of Items:**  
  - **Type of Work:** ☒ New  ☐ Repair  ☐ Rework
  - **Tested:** 4  
  - **Accepted:** 3  
  - **Rejected:** 1

**Inspector:** Skip Szurek  
**Level:** II  
**Accepted by:** Skip Szurek
Performed magnetic particle examination of two samples of hanger bar, labeled 3A and 3B. This examination was done after fatigue testing. One longitudinal indication was noted on bottom side of plate 3B between holes labeled X and Y. No other indications not previously noted were detected. Refer to attached Nondestructive Examination Report for additional details.

To the best of our knowledge, all items inspected today are in conformance with approved plans and specifications.

Inspector: Skip Szurek
Reviewed By:

Michael S. Dolder, P.E.
Vice President
Nondestructive Examination Report

Project No.: L13023  Date: 5/17/13
Project: WSDOT Live Load Hangar Bar
Type of Inspection: ☑ MT  ☐ PT

NDE Procedure: MTE-AWS  Revision No.: 0  Acceptance Standard: AWS D1.1
Material Type: Carbon Steel  Surface Condition: ☑ As Welded  ☐ Machined  ☐ Ground
Material Temp: 60's  Heat Treatment: ☐ Before  ☐ After

Magnetic Particle
Equip. Manufacturer: Parker  Insp. Method: ☑ Dry  ☐ Wet  ☑ Visible  ☐ Fluorescent
Model: B300  Current: ☑ AC  ☐ DC  Amperage: 6A
Serial No.: 16766  Cal Date: 1/13  Prod Spacing: 4"  ☐ Head Shot  ☐ Coil

Dye Penetrant
Penetrant Manufacturer: ___________________________  Test Method: ☐ Solvent Removable  ☐ Visible
Dwell Time: ______  Development Time: ______  ☐ Water Washable  ☐ Fluorescent

<table>
<thead>
<tr>
<th>Weld or Part No.</th>
<th>Location</th>
<th>Accept</th>
<th>Reject</th>
<th>Remarks</th>
</tr>
</thead>
<tbody>
<tr>
<td>3A</td>
<td>four sides of plate</td>
<td>☑</td>
<td>☑</td>
<td>informational inspection</td>
</tr>
<tr>
<td>3B</td>
<td>four sides of plate</td>
<td>☑</td>
<td>☑</td>
<td>informational inspection</td>
</tr>
</tbody>
</table>

Type of Work: ☑ New  ☐ Repair  ☐ Rework
Tested: 2  Accepted: 2  Rejected: 0

Inspector: Skip Szurek  Level: II  Accepted by: Skip Szurek