Research Report Agreement T4118, Task 92 WSF Bearings Tests

FATIGUE AND STRENGTH TESTS OF HEAT-STRAIGHTENED FERRY LOADING BRIDGE HANGER BARS

by

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16. ABSTRACT:

Tests were conducted on heat-straightened and/or bent live load hanger bars used in loading bridges in the Washington State Ferry (WSF) system. Both fatigue and ultimate strength tests were conducted. The study found that when heat-straightened three times, the hanger bars have a fatigue life that exceeds their design life. The data indicated that additional heat-straightening may be possible without concern for reducing the fatigue life. The yield strength of the hanger bars was found to be unaffected by either heat-straightening or by initial bending deformations. In both cases the hanger bar yield strength exceeded nominal values. The ultimate strength was somewhat reduced by the presence of initial bending deformation.

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EXECUTIVE SUMMARY

Objectives

The objective of this study was to determine whether heat-straightened and/or bent ferry loading bridge hanger bars have adequate fatigue life and ultimate strength.

Background

The bridges used to load vehicles onto Washington State ferries are supported on one end by hanger bars. These bars carry bridge loads in tension but can buckle in compression as the ferry rises with rising tides while at the dock. Washington State Ferries (WSF) engineers heat-straighten the buckled bars and return them to service. However, it is unclear whether the bars can be heat-straightened three times and safely reused. It is also unclear to WSF engineers what the ultimate tensile capacity of the plastically buckled bars is.

Research Activities

Two sets of tests were conducted on heat-straightened hanger bars. First, bars that had been heatstraightened three times were tested under fatigue loading with the amplitude of the varying loading near the design load for the bars, determined by the live truck loads on the bridge. Second, several hanger bars, heat-straightened two or three times or cold bent to 5 degrees, were tested in tension to failure to determine their ultimate strength.

Conclusions

Fatigue tests demonstrated that hanger bars heat-straightened three times have a fatigue life of at least 3 million cycles at a load range of 50 kips (10 kips tension to 60 kips tension). The ultimate strength tests demonstrated that the bars were able to reach the yield capacity of the net section regardless of the heat straightening or initial out-of-straightness. Ultimate hanger bar strength was not affected by heat-straightening, but initial out-straightness did reduce the ultimate capacity slightly. Results from all tests indicated that, for loads within the range used for testing, bars may be safely heat-straightened at least three times—and likely more—and returned to service.

INTRODUCTION, MOTIVATION AND OBJECTIVES

The bridges used to load vehicles onto Washington State ferries are supported on one end by hanger bars. Figures 1, 2, and 3 show a typical loading bridge and hanger bar. These bars carry bridge loads in tension, but they can buckle in compression as the ferry rises with rising tides while at the dock or when workers adjust the bridge without removing the pins. Washington State Ferries (WSF) engineers then heat-straighten the bars and return them to service. However, it is unclear whether the bars can be heat-straightened three times and safely reused.



Figure 1. Loading Bridge Cross-Section with Hanger Bars Labeled.

The objective of this research was to determine whether heat-straightened ferry loading bridge hanger bars have adequate fatigue life and ultimate strength. To achieve this objective, the



researchers carried out fatigue and ultimate strength tests in the University of Washington (UW) Structural Research Laboratory (SRL).

Figure 2. Loading Bridge Elevation with Hanger Bar Labeled.



Figure 3. Hanger Bar Detail

FATIGUE TESTING SETUP AND RESULTS

Test Set-Up

Two hanger bar specimens were tested under fatigue loading in a 110-kip fatigue test frame in the SRL. The test set-up is shown in Figure 4a. The specimens were 40 in. long and had three of the oval shaped holes shown in Figure 3, with 4 in. of overhang on each end. A typical specimen is shown in Figure 4b. Each specimen had been heat-straightened three times, and they were named 3A and 3B. The specimens were connected to the test frames by using a series of plates and pins. The pins that were used in bearing against the hanger bars were identical to those used in the ferry loading bridges to ensure that the stress distribution in the tests closely matched that expected in the field.



Figure 4. (a) Fatigue Test Setup (b) Fatigue Specimen

Mayes Testing Inc. conducted magnetic particle testing on both the fatigue specimens before and after testing to look for cracks. Some surface cracks were noted before testing but were likely the result of corrosion and did not grow during the tests. The inspection reports from Mayes Testing Inc. are included in the Appendix.

Both specimens were subjected to sinusoidal cyclic fatigue loading with peaks at 60 kips and 10 kips of tension at a rate of 3 Hz. The loading was conducted around the clock, and emergency switches were utilized to sense a failure and stop the hydraulic system. This loading protocol was agreed to by the SRL staff and the WSF engineers. They determined that the specimens should be subjected to 3 million cycles of loading, at which point the tests would be stopped if no failure occurred.

Experimental Results

Both specimens were loaded to 3 million cycles without failure. Post-test magnetic particle inspections conducted by Mayes Testing Inc. showed no signs of cracking at the hanger bar net section or in the regions of heat-straightening. Both specimens were then reused in the ultimate strength tests described below. Although twice heat-straightened specimens were also prepared, they were not tested in fatigue because the specimens that had been straightened three times performed well. Instead, they were tested for ultimate strength as described below. The design life for these bars is 10 years, and according to WSF engineers, the design life results in a loading of 1.3 million cycles at the tested stress range. Therefore, the experimental results indicated that the bars heat-straightened three times have ample fatigue life for their intended design life.

ULTIMATE STRENGTH TESTING SET-UP AND RESULTS

Test Set-up

Ultimate strength tests were conducted at the top of the SRL's 2.4-million-pound capacity Universal Testing Machine (UTM). Two lengths of specimens were tested: (1) nominally 10-ft-long bars, one that had been heat-straightened multiple times and two that were installed bent to simulate their condition after buckling, and (2) 40-in.-long bars that were used (or designed to be used) in the fatigue test set-up. Figure 5 shows the two types of specimens installed in the UTM. The material for all bars was either A36 or an unknown older steel.



Figure 5. Ultimate Strength Test Setup (a) Nominal 10 ft Specimen (b) 40 in. Specimen

The set-up utilized pin connections at each end of the hanger bars, and the pins used were identical to those used in the field. Loading was applied slowly to each specimen and continued

until failure. Only the load was recorded during the tests. However, the tests were conducted under displacement control, i.e., the displacement of the UTM crosshead was used to control loading during the test. This crosshead displacement was applied at a uniform, slow rate. Thus, plotting the force applied versus time would indicate when the specimens began to yield, since there is a linear relationship between time and displacement.

Experimental Results

Each specimen exhibited reasonable ductility before fracturing either at a net section area adjacent to a slotted hole within the length of the specimen or at the net section where the pins connected to the specimens. After the tests, signs of yielding at all net section areas adjacent to the slotted holes were visible. An example is shown in Figure 6a. An example of the typical net section fracture that occurred after significant inelastic deformation is shown in Figure 6b.



(a)

(b)

Figure 6. (a) Example of typical slotted hole yielding and deformation. (b) Example of typical net section fracture.

Table 6 lists the details of each specimen. Specimens Ult1 and Ult2 were 10 ft long and were tested after being bent (Figure 5a). Specimen Ult3 had been heat-straightened three times. Specimens F3A and F3B were 40 in. long and had been subjected to 3 million cycles of fatigue loading as described above. Specimens F2A and F2B were 40 in. long and had been heat-straightened twice. As noted above, all specimens exhibited ductile behavior, and the two bent specimens did not fracture at the bend but rather at a different net section (at the pin connection or one slotted hole away from the pin connection).

Figure 7 shows the force versus time curves for each tested specimen, where the names correspond to the information in Table 1. As noted above, a constant rate of crosshead displacement was used to control the test so that the load versus time curves would be similar to load deformation curves. Figure 7 shows that each specimen had a clear yield point at which the slope of the curve changed. This was followed by ductile inelastic deformation and eventual fracture at a net section adjacent to a slotted hole. Note that the two long bent specimens showed a smaller initial stiffness as the bend in the bar was straightened. Also note that those two specimens had slightly lower ultimate strengths than the other specimens, although the yield strength was similar.

By using the curves of Figure 7, the yield and ultimate strengths of each specimen were determined, and the results are given in Table 1. The table also shows the nominal yield and ultimate strengths, assuming a yield stress of 36 ksi and an ultimate stress of 58 ksi each, times the net area at a long slotted hole of 9.5 in^2 . A comparison of the nominal and experimentally obtained values indicated that the inelastic buckling deformation, heat-straightening (up to three times), and fatigue loading generally did not affect the yield strengths of the bars. The data in Table 1 do indicate that the bars tested in the bent configuration had somewhat lower ultimate strengths, producing effective ultimate stresses of 56.7 for Specimen Ult1 and 60 ksi for Specimen Ult2. The former was slightly lower than the minimum ultimate stress for A36 of 58 ksi. It is unclear why these specimens exhibited a lower ultimate strength, especially given that the fractures occurred far from the bends. Heat-straightening and fatigue loading were not found to affect the ultimate strength.



(g) Specimen Ult3 Figure 7. Load vs. Time Curves for Ultimate Strength Tests

Specimen ID	Specimen Length	Specimen Details	Yield Strength ¹ (kips)	Ultimate Strength ² (kips)
F2A	40 in.	Heat-straightened two times, loaded only for ultimate strength, A36 steel	408	651
F2B	40 in.	Heat-straightened two times, loaded only for ultimate strength, A36 steel	405	638
F3A	40 in.	Heat-straightened three times, loaded in fatigue before ultimate strength, A36 steel	402	661
F3A	40 in.	Heat-straightened three times, loaded in fatigue before ultimate strength, A36 steel	404	668
Ult1	10 ft.	Bent bar at approximately 10°, loaded only for ultimate strength, A36 steel	407	539
Ult2	10 ft.	Bent bar at approximately 10°, loaded only for ultimate strength, A36 steel	402	570
Ult3	10 ft.	Heat-straightened, loaded only for ultimate strength, unknown steel	430	698
Nominal	-	36 ksi yield stress, 58 ksi ultimate stress	342	551

¹ Yield strength estimated as point of significant change in slope in Figure 7 plots. ² Maximum force obtained during test.

CONCLUSIONS

The testing program demonstrated that hanger bars heat-straightened up to three times are able to

- resist 3 million cycles of fatigue loading with a range of 50 kips
- develop good ductility
- achieve yield strengths consistent with the yield stress of the original material, and
- achieve ultimate strengths consistent with the tensile stress of the original material.

Hanger bars with large bends from inelastic buckling are also able to achieve yield strengths consistent with the yield stress of the material, but they were observed in one case to have an ultimate strength slightly lower than what would be expected by developing the tensile stress of the material over the net section area.

LABORATORY DISCLAIMER STATEMENT:

The Structural Research Laboratory provides commercial testing services. These services are limited to testing and data collection. The results are valid at the time the test occurs on the specific specimens tested. The engineering response of similar items is not within the scope of the testing agreement. The SRL staff, the Department of Civil and Environmental Engineering, the College of Engineering, and the University of Washington disclaim any and all liability for any personal or property damage or loss as a result of use of the test results.

APPENDIX

Below are inspection reports from Mayes Testing Engineers, Inc. They performed magnetic particle testing on the hanger bars before fatigue loading (inspection on April 2, 2013) to check for initial cracks and following fatigue loading (inspection on May 17, 2013) for Specimen F3A and F3B (denoted 3A and 3B in the inspections reports). As shown, no indications of significance were found. Small indications parallel to the direction of the applied load were found, but they were not due to the fatigue loading applied but instead may have been related to corrosion or material imperfections.

MAYES TESTING ENGINEERS, INC.

 Project No.
 L13023

 Project
 WSDOT Live Load Hangar Bar

 Address
 More Hall, University of Washington, Seattle, WA

 Permit No.
 N/A

 Bldg Dept.
 N/A

Owner University of Washington

Record No.001Date4/9/2013Weather(indoors)InspectionWeldingSample(s)N/A

Seattle Office 20225 Cedar Valley Road Suite 110 Lynnwood, WA 98036 ph 425.742.9360 fax 425.745.1737

Tacoma Office 10029 S. Tacoma Way Suite E-2 Tacoma, WA 98499 ph 253.584.3720 fax 253.584.3707

Portland Office 7911 NE 33rd Drive Suite 190 Portland, OR 97211 ph 503.281.7515 fax 503.281.7579

Performed magnetic particle examination of four load bar plates, 2A, 2B, 3A, and 3B. This inspection was performed to determine if there were any flaws prior to fatigue testing. Noted flaw in bottom of plate 3A at X end. Notified Jeff Berman and Vince with UW.



Fig. 1. Crack noted in bottom of 3A plate at X end

To the best of our knowledge, all items inspected today are in conformance with approved plans and specifications.

Inspector: Skip Szurek

Reviewed By:

8 de

Michael S. Dolder, P.E. Vice President

Page 1 of 1

Page 1 of 1

MAYES TESTING ENGINEERS, INC				Nondestructive Examination Report						
	Valley Road, Suite 110 Ph 425.742.9360 /A 98036 Fax 425.745.1737 F			Project No.: L13023 Date: 4/9/13						
10029 S. Tacoma Way, Su	ite E-2	Ph	253 584 3720		50 10 10 10 10 10 10 10 10 10 10 10 10 10 1			Verse Parage	ingar Bar	-
7911 NE 33 rd Drive Suite 190 Ph 503 281 7515			100							
NDE Procedure: M				on No.: 0		Acce	eptance S	tandard:	AWS D1.1	
Material Type: Cart										d 🗌 Ground
Material Temp: 50's					Heat Treatment: Before After					
Magnetic Particle Equip. Manufacturer:	Parke	r			Insp. M	ethod	: 🛛 Dry	Wet	⊠ Visible [Fluorescen
Model: B300										
Serial No.: 16766										
Dye Penetrant Penetrant Manufactur						thod:				
Dwell Time:	-							er Washat		orescent
Weld or Part No.		L	ocation	Accept	Reject			R	emarks	
2A	four s	ides o	f plate	X		infor	rmational	inspection		
2B	four s	ides o	f plate	X		infor	rmational	inspection		
ЗA	top ar	nd bot	h sides	X		infor	mational	inspection		
	bottom			X 5" longitudinal crack at X end				X end		
3B	3B four sides of plate X		X		infor	mational	inspection			
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Type of Work: 🛛 Ne	ew 🗌	Repa	ir 🗌 Rework	No. of It Tested:			Accepted	: _3	Rejected	: 1
Inspector: Skip Szur	ek			Level:	Ш	Acce	epted by:	Skip Szu	ırek	

MTE 1520-2C, Rev 0, 7/2/07

MAYES TESTING ENGINEERS, INC.

 Project No.
 L13023

 Project
 WSDOT Live Load Hangar Bar

 Address
 More Hall, University of Washington, Seattle, WA

 Permit No.
 N/A

 Bldg Dept.
 N/A

Owner University of Washington

Record No.002Date5/17/2013Weather(indoors)InspectionWeldingSample(s)N/A

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Portland Office 7911 NE 33rd Drive Suite 190 Portland, OR 97211 ph 503.281.7515 fax 503.281.7579

Performed magnetic particle examination of two samples of hanger bar, labeled 3A and 3B. This examination was done after fatigue testing. One longitudinal indication was noted on bottom side of plate 3B between holes labeled X and Y. No other indications not previously noted were detected. Refer to **attached** Nondestructive Examination Report for additional details.

To the best of our knowledge, all items inspected today are in conformance with approved plans and specifications.

Inspector: Skip Szurek

Reviewed By:

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Michael S. Dolder, P.E. Vice President

Page	1	of	1		

MAVEC				
	ING ENGINEERS, INC		No	ndestructive Examination Report
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10029 S. Tacoma Way, Su Tacoma, WA 98499	ite E-2 Ph 253.584.3720 Fax 253.584.3707	F	Project:	WSDOT Live Load Hangar Bar
7911 NE 33 rd Drive, Suite 1 Portland, OR 97211	90 Ph 503.281.7515 Fax 503.281.7579	1	Type of I	nspection: 🛛 MT 🗌 PT
NDE Procedure: MT	E-AWS Revisio	n No.: 0		Acceptance Standard: AWS D1.1
Material Type: Cart	oon Steel		Surface	Condition: 🛛 As Welded 🗌 Machined 📋 Ground
Material Temp: 60's			Heat Tr	eatment: 🗌 Before 🗌 After
Magnetic Particle Equip. Manufacturer:	Parker		Insp. Me	ethod: 🖂 Dry 🗌 Wet 🖾 Visible 🔲 Fluorescent
Model: B300			Current	AC DC Amperage: 6A
Serial No.: 16766	Cal Date: _1/1:	3	Prod Sp	acing: _4"
Dye Penetrant Penetrant Manufactur	er:		Test Me	thod: Solvent Removable Visible
	Development Time:			
Weld or Part No.	Location	Accept	Reject	Remarks
3A	four sides of plate	X		informational inspection
3B	four sides of plate	x		informational inspection
			-	
		-	5	
·			o	
Type of Work: 🛛 Ne	ew 🗌 Repair 🗌 Rework	No. of It Tested:		Accepted: 2 Rejected: 0
Inspector: Skip Szur	ek	Level:	П	Accepted by: Skip Szurek